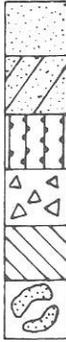


**SUBSOIL
INVESTIGATIONS**



SOIL MECHANICS DRILLING CORP.

3770 MERRICK ROAD • SEAFORD, L. I., NEW YORK 11783
(516) 221-2333 • FAX (516) 221-0254

March 17, 2020

VHB
100 Motor Parkway, Suite 350
Hauppauge, NY 11788
Att: Suzy Silverstein

Re: Tam O'Shanter Golf Club
Brookville, NY
Our Job #20-065

Dear Ms. Silverstein:

Forwarded herewith are the results of the test borings drilled at the above referenced site.

The purpose of the subsurface investigation was to determine the nature and extent of the underlying soil deposits and determine the structural engineering and drainage characteristics of the soil at the site. A total of eight (8) test borings were drilled, two to a depth of 37 feet each due to the high silt content encountered at borings B-1 and B-2 and six to a depth of 25 feet each, using truck mounted drilling equipment at the locations shown on our Boring Location Plan. The borings were advanced using hollow stem auger casing. Sample recovery was obtained with a 2" diameter, 2'0" long split spoon sampler was advanced into the subsurface by the use of an automatic 140 lb. hammer with a 30" drop. From the drops of the hammer, blow counts required to advance the split spoon sampler over each 6" intervals were recorded and is shown on the boring logs. Continuous split spoon samples were taken for the top 12 feet then every 5 feet thereafter to the final depths of the borings. A written description of the recovered soil samples per our geologist's visual identification of same is also presented on the logs.

The CME automatic hammer operates with an efficiency of approximately 90%. The original conventional use of rope, cathead and drop weight, on the other hand, operates with an efficiency of approximately 60%. As a consequence, the standard penetration test results obtained using the CME auto-hammer are on the order of two-thirds the value that would have been obtained had the original rope and cathead method been used. This is significant if you are using design charts for soil strength parameters based on historical data associated with the rope and cathead method. If so, you should adjust our data accordingly.

Our investigation revealed that the areas drilled are blanketed by from 2 to 4 feet of loam and soil fill, underlain, generally, by a moderately dense to dense coarse to fine sand with traces of silt and gravel extending to the deepest depths drilled.

**TEST BORINGS • GROUND WATER DETERMINATIONS • FOUNDATION RECOMMENDATIONS • HOLLOW STEM AUGER BORINGS
LABORATORY ANALYSES • CONTROLLED LANDFILL • DIAMOND CORE DRILLING • SAND & GRAVEL PROSPECTING
BEARING VALUES • WELL POINT INSTALLATIONS • ENGINEERING SUPERVISION • PERCOLATION TESTS
SANITARY INVESTIGATIONS • UNDISTURBED SAMPLING • TEST PITS • TOP SOIL ANALYSES**

VHB
Att: Suzy Silverstein

March 17, 2020
Page 2

Please note at borings B-1 and B-2 the silt content was much larger than the other six borings therefore those borings were extended to find the free draining sand.

With the exception of perched water encountered at borings B-1 and B-7, no ground water was encountered in any of the other borehole locations at the time the work was done.

Eight (8) percolation tests were performed and the results are as shown on our drawing.

We have not been informed of the construction details of this project. However, we can make the following recommendations at this time:

All soils below the loam will satisfactorily support foundation loads of 3 tons per square foot.

All soils classified as "SP" exhibit excellent drainage characteristics.

If areas have to be removed because of the loam, we recommend replacing with a supervised controlled fill consisting of a clean inorganic material, placed in lifts not to exceed 12 inches in depth, and be compacted, under independent supervision, to a minimum of 95% of the Modified Proctor maximum dry density at optimum moisture content per ASTM D 1557. Soils to be used for controlled fill shall have less than 10% passing a #200 sieve with no particle size larger than 2 inches. The natural "SP" soil can be used for the controlled fill operation if needed.

Frost penetration in this area is 3 feet. All exterior foundations must have a minimum of 3 foot of cover.

Liquefaction is not a design factor.

The soils generated by this investigation best fit that of seismic Site Class "D" in accordance with Table 1513.5.5 of the New York State Building Code.

Soil samples recovered during drilling operations will be stored in our lab for a period of 30 days after which they will be destroyed. During this period we will deliver these samples to any prescribed location upon request.

SOIL MECHANICS DRILLING CORP.

3770 MERRICK ROAD • SEAFORD, L. I., NEW YORK 11783
(516) 221-2333 • FAX (516) 221-0254

VHB
Att: Suzy Silverstein

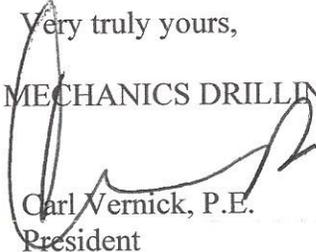
March 17, 2020
Page 3

If after you examine the enclosed you have any further questions, please feel free to call and discuss them with us.

Billing is enclosed.

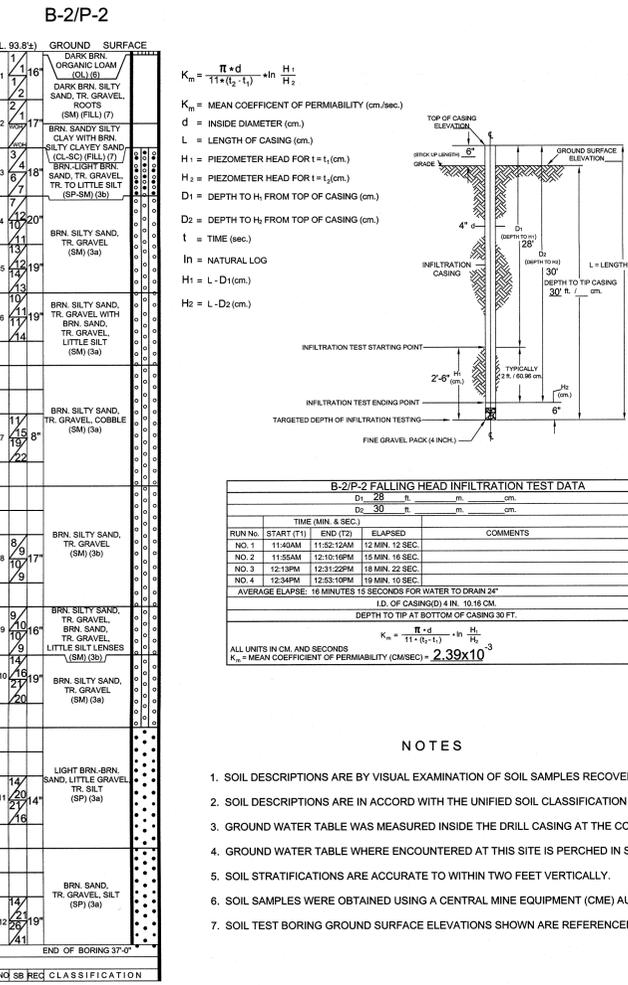
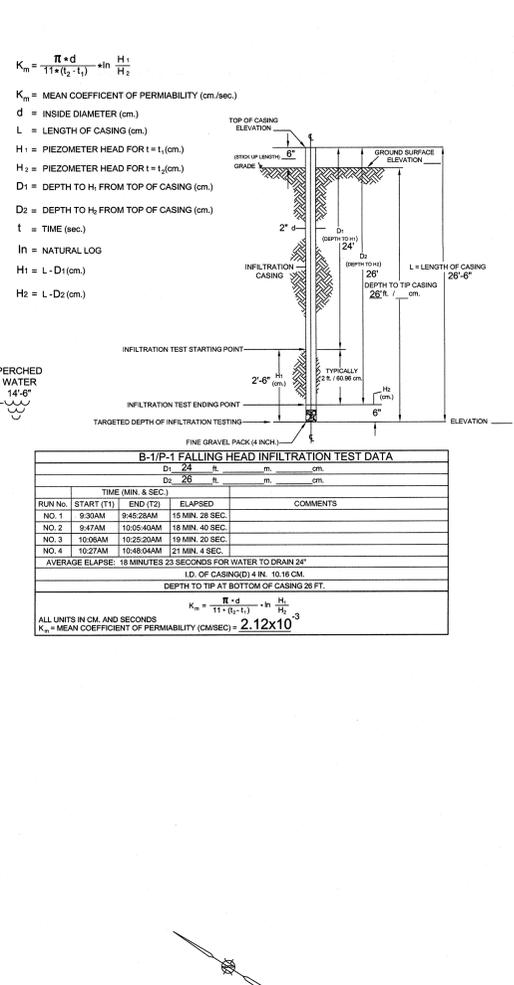
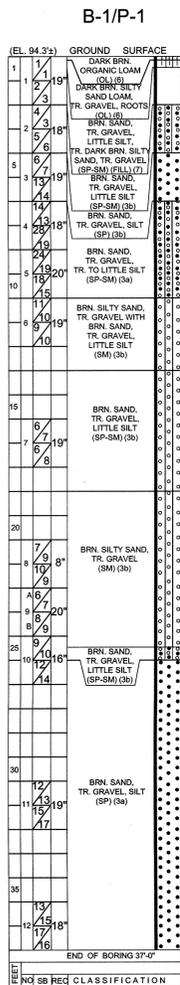
Very truly yours,

SOIL MECHANICS DRILLING CORP.

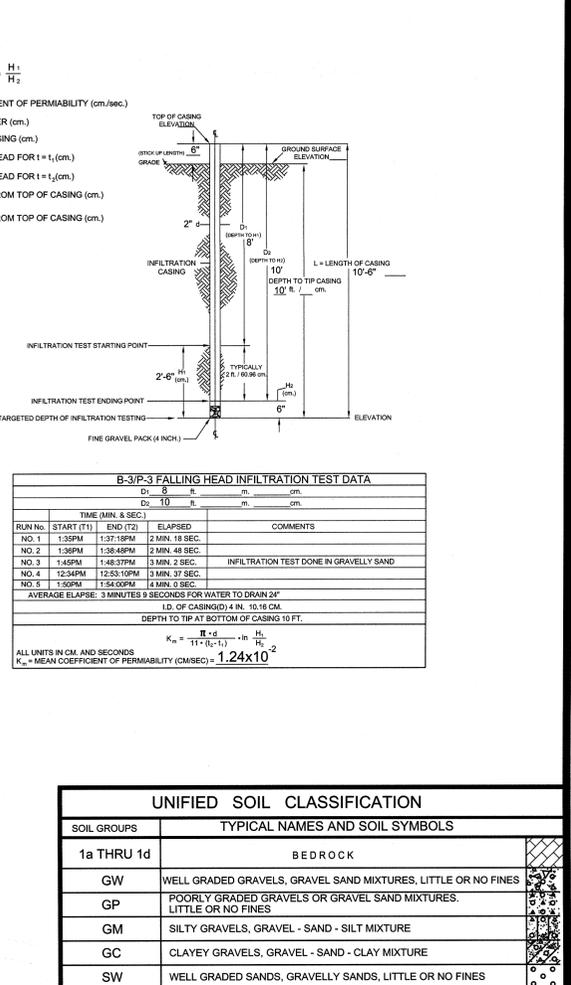


Carl Vernick, P.E.
President

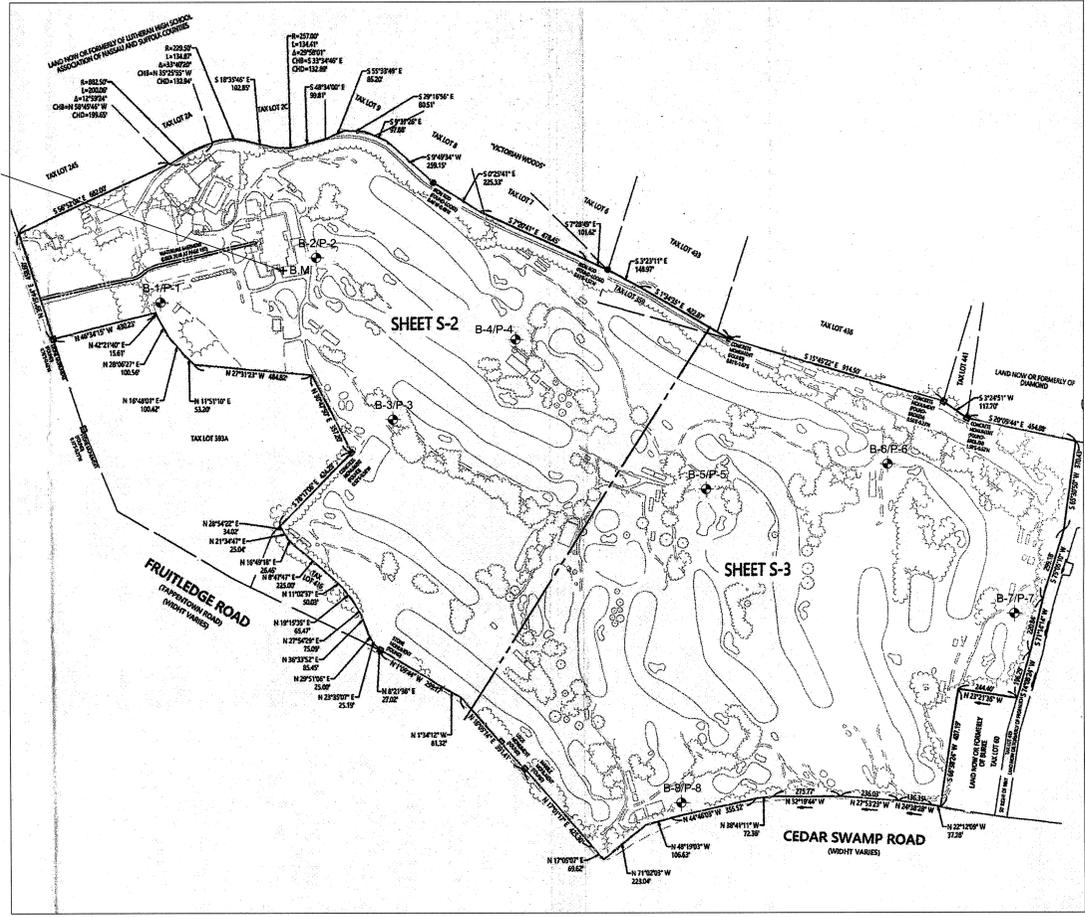
CV:mlf
Encls.



- NOTES**
- SOIL DESCRIPTIONS ARE BY VISUAL EXAMINATION OF SOIL SAMPLES RECOVERED DURING DRILLING OPERATIONS.
 - SOIL DESCRIPTIONS ARE IN ACCORD WITH THE UNIFIED SOIL CLASSIFICATION SYSTEM.
 - GROUND WATER TABLE WAS MEASURED INSIDE THE DRILL CASING AT THE COMPLETION OF EACH BOREHOLE.
 - GROUND WATER TABLE WHERE ENCOUNTERED AT THIS SITE IS PERCHED IN SOME AREAS AND NOT CONSIDERED RELIABLE.
 - SOIL STRATIFICATIONS ARE ACCURATE TO WITHIN TWO FEET VERTICALLY.
 - SOIL SAMPLES WERE OBTAINED USING A CENTRAL MINE EQUIPMENT (CME) AUTOMATIC TRIP HAMMER.
 - SOIL TEST BORING GROUND SURFACE ELEVATIONS SHOWN ARE REFERENCED TO TOP OF GRATE AT DRY INLET AT ASSUMED B.M. EL. 100.00'.



SOIL BORING NO.	SOIL TEST BORING COORDINATES ONLY
B-1/P-1	N 40°- 48'-997 W 073°- 33'-253
B-2/P-2	N 40°- 48'-919 W 073°- 33'-151
B-3/P-3	N 40°- 48'-824 W 073°- 33'-233
B-4/P-4	N 40°- 48'-774 W 073°- 33'-119
B-5/P-5	N 40°- 48'-620 W 073°- 33'-147
B-6/P-6	N 40°- 48'-532 W 073°- 33'-028
B-7/P-7	N 40°- 48'-393 W 073°- 33'-075
B-8/P-8	N 40°- 48'-516 W 073°- 33'-399



SOIL GROUPS	TYPICAL NAMES AND SOIL SYMBOLS
1a THRU 1d	BED ROCK
GW	WELL GRADED GRAVELS, GRAVEL SAND MIXTURES, LITTLE OR NO FINES
GP	POORLY GRADED GRAVELS OR GRAVEL SAND MIXTURES, LITTLE OR NO FINES
GM	SILTY GRAVELS, GRAVEL - SAND - SILT MIXTURE
GC	CLAYEY GRAVELS, GRAVEL - SAND - CLAY MIXTURE
SW	WELL GRADED SANDS, GRAVELLY SANDS, LITTLE OR NO FINES
SP	POORLY GRADED SANDS OR GRAVELLY SANDS, LITTLE OR NO FINES
SM	SILTY SANDS, SAND - SILT MIXTURES
SC	CLAYEY SANDS, SAND - CLAY MIXTURES
ML	INORGANIC SILTS, VERY FINE SANDS, CLAYEY SILTS, SLIGHT PLASTICITY
CL	INORGANIC CLAYS OF LOW TO MEDIUM PLASTICITY, GRAVELLY CLAYS SANDY CLAYS, SILTY CLAYS
OL	ORGANIC SILTS AND ORGANIC SILTY CLAYS OF LOW PLASTICITY
MH	INORGANIC SILTS, MICACEOUS OR DIATOMACEOUS FINE SANDY OR SILTY SOILS, ELASTIC SILTS
CH	INORGANIC CLAYS OF HIGH PLASTICITY, FAT CLAYS
OH	ORGANIC CLAYS OF MEDIUM TO HIGH PLASTICITY, ORGANIC SILTS
Pt	PEAT AND OTHER HIGHLY ORGANIC SOILS

CLASS OF MATERIALS (Notes 1 and 3) *	ALLOWABLE SOIL BEARING PRESSURES, N.Y.C. BLDG. CODE TABLE 1804.1	
	MAXIMUM ALLOWABLE FOUNDATION PRESSURE (TSF)	MAXIMUM ALLOWABLE FOUNDATION PRESSURE (PSF)
1. BEDROCK (NOTES 2 AND 7) *		
1a HARD SOUND ROCK - GNEISS, DIABASE, SCHIST	60	5,746
1b MEDIUM HARD ROCK - MARBLE, SERPENTINE	40	3,830
1c INTERMEDIATE ROCK - SHALE, SANDSTONE	20	1,915
1d SOFT ROCK - WEATHERED ROCK	8	766
2. SANDY GRAVEL AND GRAVEL (GW, GP) (NOTES 3, 4, 8, and 9) *	10	958
2a DENSE	6	575
2b MEDIUM	3	287
3. GRANULAR SOILS (GC, GM, SW, SP, SM, & SC) (NOTES 4, 5, 8, and 9) *	6	575
3a DENSE	3	287
3b MEDIUM	3	287
4. CLAYS (SC, CL, & CH) (NOTES 4, 6, 8, and 9) *	5	479
4a HARD	3	287
4b STIFF	3	287
4c MEDIUM	2	192
5. SILTS & SILTY SOILS (ML and MH) (NOTES 4, 8, and 9) *	3	287
5a DENSE	1.5	144
5b MEDIUM	1.5	144
6. ORGANIC SILTS, ORGANIC CLAYS, PEATS, SOFT CLAYS, LOOSE GRANULAR SOILS AND VARIED SILTS	SEE 1804.2.1 *	SEE 1804.2.1 *
7. CONTROLLED AND UNCONTROLLED FILLS	SEE 1804.2.2 OR 1804.2.3 *	SEE 1804.2.2 OR 1804.2.3 *

COMPACTION RELATED TO SPOON BLOWFOOT	STANDARD PENETRATION TEST-ASTM D 1586	N*	SP (SPOON BLOW PER FOOT)	
			SOFT	CLAY
LOOSE	LESS THAN 10	SOFT	LESS THAN 4	CLAY
MEDIUM	10 TO 30	MEDIUM	4 TO 6	CLAY
DENSE	GREATER THAN 31	STIFF	GREATER THAN 6 TO 30	CLAY
		HARD	GREATER THAN 30	CLAY

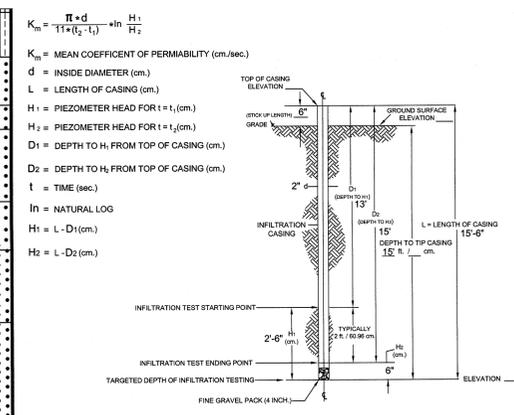
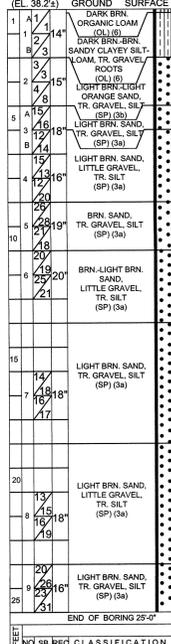
THE LIABILITY OF SOIL MECHANICS DRILLING CORP., ITS OFFICERS OR EMPLOYEES FOR ERRORS, OMISSIONS OR NEGLIGENCE RESULTING IN PERSONAL INJURIES, PROPERTY DAMAGE OR ANY CONSEQUENTIAL DAMAGES, IS LIMITED TO THE AMOUNT OF THE FEE PAID FOR THIS REPORT. THE RETENTION OR USE OF ANY PART OF THIS REPORT WILL CONSTITUTE AN ACCEPTANCE OF THE LIMITATION OF LIABILITY. IF THIS IS UNACCEPTABLE TO THE CLIENT, MUST NOTIFY SOIL MECHANICS DRILLING CORP. IN WRITING BY CERTIFIED MAIL WITHIN SEVEN DAYS FROM THE DATE OF RECEIPT. THE FEE CHARGED FOR THIS REPORT IS BASED ON THIS LIMITATION OF LIABILITY WHICH IS THE ESSENCE OF THIS AGREEMENT. IF THE CLIENT WANTS A HIGHER LIMITATION OF LIABILITY, SOIL MECHANICS DRILLING CORP. WILL NEGOTIATE ONE, BASED UPON A HIGHER FEE BEING CHARGED FOR THE ADDITIONAL ASSUMPTION OF LIABILITY. SOIL MECHANICS DRILLING CORP., ITS OFFICERS OR EMPLOYEES, HAVE NO LIABILITY OR RESPONSIBILITY TO PERSONS OTHER THAN THE CLIENT FOR WHOM THIS REPORT WAS PREPARED. ANYONE OTHER THAN OUR CLIENT, RELIES ON THIS AT THEIR OWN RISK.

SOIL MECHANICS DRILLING CORP.
subsoli investigations
 3770 MERRICK ROAD * SEAFORD, NEW YORK 11783 * 516 221-2333

SUBSURFACE INVESTIGATION
 TAM O'SHANTER GOLF CLUB
 74 FRUITLEDGE ROAD
 GLEN HEAD, NEW YORK

VERTICAL BORING SCALE: 3/8" = 1'-0" UNLESS NOTED OTHERWISE
 DATES OF BORINGS: MARCH 9, 10 & 11, 2020
 DRAWING DATE: MARCH 16, 2020
 DRAWN BY: NAW
 CHECKED BY: CV
 REVISION DATE:
 DRAWING NUMBER: 20L065.8
 SHEET 1 OF 2

B-4/P-4

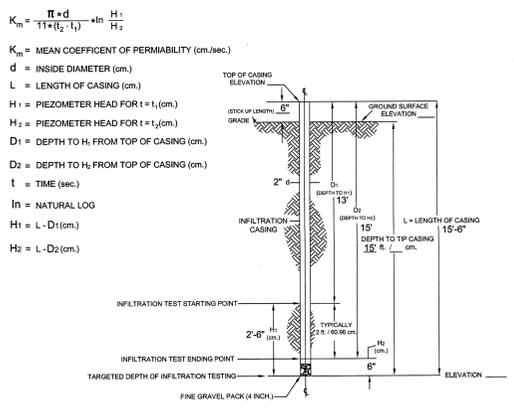
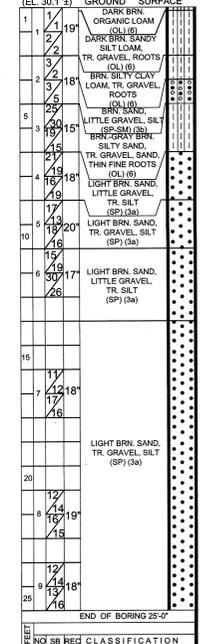


B-4/P-4 FALLING HEAD INFILTRATION TEST DATA

TIME (MIN. & SEC.)	START (T1)	END (T2)	ELAPSED	COMMENTS
NO. 1	9:53AM	9:57:04AM	4 MIN. 4 SEC.	
NO. 2	9:58AM	10:02:44AM	4 MIN. 44 SEC.	
NO. 3	10:04AM	10:09:18AM	5 MIN. 18 SEC.	
NO. 4	10:11AM	10:16:42AM	5 MIN. 42 SEC.	

AVERAGE ELAPSE: 4 MINUTES 57 SECONDS FOR WATER TO DRAIN 24"
 I.D. OF CASING(D) 4 IN. 10.16 CM.
 DEPTH TO TIP AT BOTTOM OF CASING 15 FT.
 $K_m = \frac{\pi \cdot d}{11 \cdot (H_1 - H_2)} \cdot \ln \frac{H_1}{H_2}$
 $K_m = \text{MEAN COEFFICIENT OF PERMIABILITY (CM/SEC)} = 7.86 \times 10^{-3}$

B-5/P-5

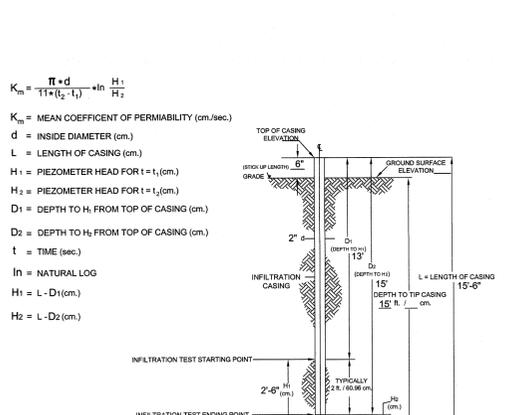
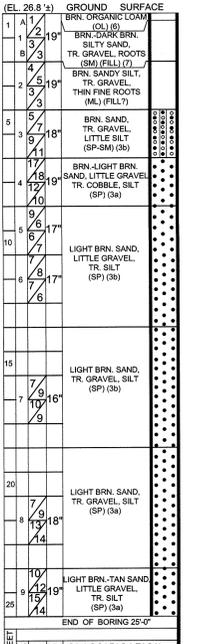


B-5/P-5 FALLING HEAD INFILTRATION TEST DATA

TIME (MIN. & SEC.)	START (T1)	END (T2)	ELAPSED	COMMENTS
NO. 1	11:45AM	11:45:24AM	5 MIN. 24 SEC.	
NO. 2	11:47AM	11:52:55AM	5 MIN. 55 SEC.	
NO. 3	11:54AM	12:00:19PM	6 MIN. 29 SEC.	
NO. 4	12:02PM	12:08:46PM	6 MIN. 44 SEC.	

AVERAGE ELAPSE: 6 MINUTES 8 SECONDS FOR WATER TO DRAIN 24"
 I.D. OF CASING(D) 4 IN. 10.16 CM.
 DEPTH TO TIP AT BOTTOM OF CASING 15 FT.
 $K_m = \frac{\pi \cdot d}{11 \cdot (H_1 - H_2)} \cdot \ln \frac{H_1}{H_2}$
 $K_m = \text{MEAN COEFFICIENT OF PERMIABILITY (CM/SEC)} = 6.35 \times 10^{-3}$

B-6/P-6

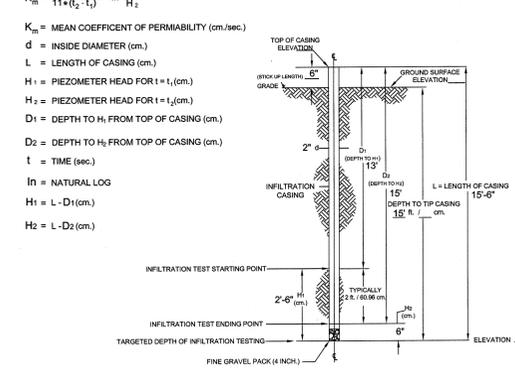
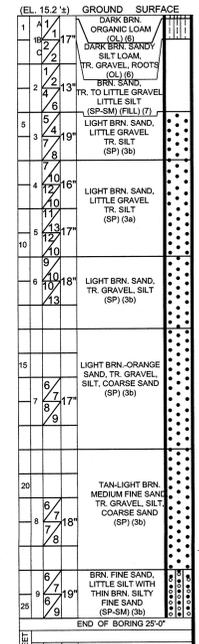


B-6/P-6 FALLING HEAD INFILTRATION TEST DATA

TIME (MIN. & SEC.)	START (T1)	END (T2)	ELAPSED	COMMENTS
NO. 1	8:25AM	8:30:20AM	5 MIN. 36 SEC.	
NO. 2	8:32AM	8:37:57AM	5 MIN. 57 SEC.	
NO. 3	8:39AM	8:45:20AM	6 MIN. 20 SEC.	INFILTRATION TEST DONE IN GRAVELLY SAND
NO. 4	8:47AM	8:53:55AM	6 MIN. 39 SEC.	

AVERAGE ELAPSE: 6 MINUTES 7 SECONDS FOR WATER TO DRAIN 24"
 I.D. OF CASING(D) 4 IN. 10.16 CM.
 DEPTH TO TIP AT BOTTOM OF CASING 15 FT.
 $K_m = \frac{\pi \cdot d}{11 \cdot (H_1 - H_2)} \cdot \ln \frac{H_1}{H_2}$
 $K_m = \text{MEAN COEFFICIENT OF PERMIABILITY (CM/SEC)} = 6.36 \times 10^{-3}$

B-7/P-7

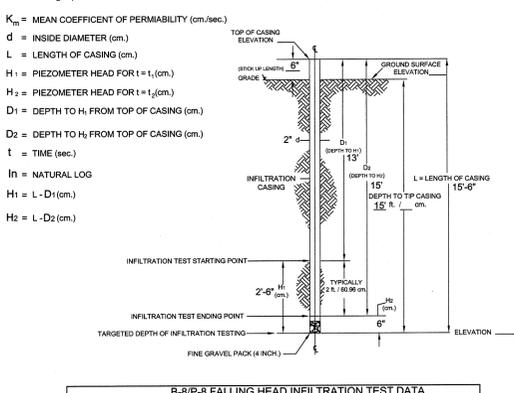
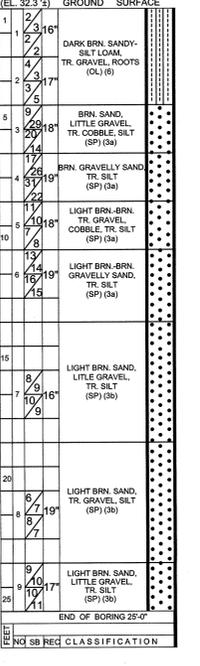


B-7/P-7 FALLING HEAD INFILTRATION TEST DATA

TIME (MIN. & SEC.)	START (T1)	END (T2)	ELAPSED	COMMENTS
NO. 1	9:11AM	9:17:27AM	6 MIN. 27 SEC.	
NO. 2	9:19AM	9:25:56AM	6 MIN. 55 SEC.	
NO. 3	9:27AM	9:34:24AM	7 MIN. 24 SEC.	INFILTRATION TEST DONE IN GRAVELLY SAND
NO. 4	9:36AM	9:43:10AM	7 MIN. 50 SEC.	

AVERAGE ELAPSE: 7 MINUTES 9 SECONDS FOR WATER TO DRAIN 24"
 I.D. OF CASING(D) 4 IN. 10.16 CM.
 DEPTH TO TIP AT BOTTOM OF CASING 15 FT.
 $K_m = \frac{\pi \cdot d}{11 \cdot (H_1 - H_2)} \cdot \ln \frac{H_1}{H_2}$
 $K_m = \text{MEAN COEFFICIENT OF PERMIABILITY (CM/SEC)} = 5.44 \times 10^{-3}$

B-8/P-8



B-8/P-8 FALLING HEAD INFILTRATION TEST DATA

TIME (MIN. & SEC.)	START (T1)	END (T2)	ELAPSED	COMMENTS
NO. 1	10:50AM	10:54:52AM	4 MIN. 52 SEC.	
NO. 2	10:56AM	11:01:44AM	5 MIN. 44 SEC.	
NO. 3	11:02AM	11:09:55AM	6 MIN. 5 SEC.	
NO. 4	11:11AM	11:17:31AM	6 MIN. 31 SEC.	

AVERAGE ELAPSE: 5 MINUTES 48 SECONDS FOR WATER TO DRAIN 24"
 I.D. OF CASING(D) 4 IN. 10.16 CM.
 DEPTH TO TIP AT BOTTOM OF CASING 15 FT.
 $K_m = \frac{\pi \cdot d}{11 \cdot (H_1 - H_2)} \cdot \ln \frac{H_1}{H_2}$
 $K_m = \text{MEAN COEFFICIENT OF PERMIABILITY (CM/SEC)} = 6.71 \times 10^{-3}$

UNIFIED SOIL CLASSIFICATION

SOIL GROUPS	TYPICAL NAMES AND SOIL SYMBOLS
1a THRU 1d	BED ROCK
GW	WELL GRADED GRAVELS, GRAVEL SAND MIXTURES, LITTLE OR NO FINES
GP	POORLY GRADED GRAVELS OR GRAVEL SAND MIXTURES, LITTLE OR NO FINES
GM	SILTY GRAVELS, GRAVEL - SAND - SILT MIXTURE
GC	CLAYEY GRAVELS, GRAVEL - SAND - CLAY MIXTURE
SW	WELL GRADED SANDS, GRAVELLY SANDS, LITTLE OR NO FINES
SP	POORLY GRADED SANDS OR GRAVELLY SANDS, LITTLE OR NO FINES
SM	SILTY SANDS, SAND - SILT MIXTURES
SC	CLAYEY SANDS, SAND - CLAY MIXTURES
ML	INORGANIC SILTS, VERY FINE SANDS, CLAYEY SILTS, SLIGHT PLASTICITY
CL	INORGANIC CLAYS OF LOW TO MEDIUM PLASTICITY, GRAVELLY CLAYS, SANDY CLAYS, SILTY CLAYS
OL	ORGANIC SILTS AND ORGANIC SILTY CLAYS OF LOW PLASTICITY
MH	ORGANIC SILTS, MICACEOUS OR DIATOMACEOUS FINE SANDY OR SILTY SOILS, ELASTIC SILTS
CH	INORGANIC CLAYS OF HIGH PLASTICITY, FAT CLAYS
OH	ORGANIC CLAYS OF MEDIUM TO HIGH PLASTICITY, ORGANIC SILTS
PT	PEAT AND OTHER HIGHLY ORGANIC SOILS

ALLOWABLE SOIL BEARING PRESSURES, N.Y.C. BLDG. CODE TABLE 1804.1

CLASS OF MATERIALS (Notes 1 and 3) *	MAXIMUM ALLOWABLE FOUNDATION PRESSURE (PSF)	MAXIMUM ALLOWABLE FOUNDATION PRESSURE (PSF)
1. BEDROCK (NOTES 2 AND 7) *		
1a HARD SOUND ROCK - GNEISS, DIABASE, SCHIST	60	5,746
1b MEDIUM HARD ROCK - MARBLE, SERPENTINE	40	3,630
1c INTERMEDIATE ROCK - SHALE, SANDSTONE	20	1,915
1d SOFT ROCK - WEATHERED ROCK	8	766
2. SANDY GRAVEL AND GRAVEL (GW, GP) (NOTES 3, 4, 6, 8, AND 9) *		
2a DENSE	10	958
2b MEDIUM	6	575
3. GRANULAR SILTS (GC, GM, SW, SP, SM, SC) (NOTES 4, 5, 6, 8, AND 9) *		
3a DENSE	6	575
3b MEDIUM	3	287
4. CLAYS (SC, CL, & CH) (NOTES 4, 6, 8, AND 9) *		
4a HARD	5	479
4b STIFF	3	287
4c MEDIUM	2	192
5. SILTS & SILTY SOILS (ML and MH) (NOTES 4, 6, 8, AND 9) *		
5a DENSE	3	287
5b MEDIUM	1.5	144
6. ORGANIC SILTS, ORGANIC CLAYS, PEATS, SOFT CLAYS, LOOSE GRANULAR SILTS AND GRAVELS	SEE 1804.2.1 *	SEE 1804.2.1 *
7. CONTROLLED AND UNCONTROLLED FILLS	SEE 1804.2.2 OR 1804.2.3 *	SEE 1804.2.2 OR 1804.2.3 *

* REFER TO SECTION 1804.2 OR NOTES FOLLOWING TABLE 1804.1 IN N.Y.C. BUILDING CODE FOR ADDITIONAL INFORMATION

COMPACTED PENETRATION TEST TO SPOON BLOW/FOOT	"N"	STANDARD PENETRATION TEST ASTM D 1586 (2" SPOON, 140# HAMMER, 30" FALL)
LOOSE	LESS THAN 10	LESS THAN 4
MEDIUM	10 TO 30	4 TO 9
DENSE	GREATER THAN 31	GREATER THAN 9 TO 30

THE LIABILITY OF SOIL MECHANICS DRILLING CORP. ITS OFFICERS OR EMPLOYEES FOR ERRORS, OMISSIONS OR NEGLIGENCE RESULTING IN PERSONAL INJURIES, PROPERTY DAMAGE OR ANY CONSEQUENTIAL DAMAGES, IS LIMITED TO THE AMOUNT OF THE FEE PAID FOR THIS REPORT. THE RETENTION OR USE OF ANY PART OF THIS REPORT WILL CONSTITUTE AN ACCEPTANCE OF THE LIMITATION OF LIABILITY. IF THE CLIENT WANTS A HIGHER LIMITATION OF LIABILITY, SOIL MECHANICS DRILLING CORP. WILL NEGOTIATE ONE, BASED UPON A HIGHER FEE BEING CHARGED FOR THE ADDITIONAL ASSUMPTION OF LIABILITY. SOIL MECHANICS DRILLING CORP., ITS OFFICERS OR EMPLOYEES, HAVE NO LIABILITY OR RESPONSIBILITY TO PERSONS OTHER THAN THE CLIENT FOR WHICH THIS REPORT WAS PREPARED. ANYONE OTHER THAN OUR CLIENT, RELIES ON THIS AT THEIR OWN RISK.

SOIL MECHANICS DRILLING CORP.
subsoil investigations
 3770 MERRICK ROAD • SEAFORD, NEW YORK 11783 • 516 221-2333

SUBSITIGATION INVESTIGATION
 TAM O'SHANTER GOLF CLUB
 74 FRUITLEDGE ROAD
 GLEN HEAD, NEW YORK

VERTICAL BORING SCALE: 1/2" = 1'-0" UNLESS NOTED OTHERWISE	DRAWING DATE: MARCH 16, 2020	DRAWING NUMBER: 20L065.8
DATES OF BORING: MARCH 9, 10 & 11, 2020	CREATED BY: NAR	REVISED DATE: CV
		SHEET 2 OF 2